

Evaluation of Two Numerical Wave Models with Inlet Physical Model

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Abstract: This paper evaluates the performance of two numerical wave models, GHOST and STWAVE, with measurements made in an idealized inlet physical model. The emphasis of this paper is on the overall performance of these models in coastal inlets. Both wave models are similar in that they employ a finite-difference method to solve the wave action conservation equation for the steady-state wave spectral transformation. However, these models differ in the computation of diffraction, reflection, wave breaking, and representation of the directional spectrum transformation. The models' performance is compared with a new set of physical model data for four different idealized inlet configurations. Wave height is measured in the physical model by a linear array of capacitance wave gauges, and wave direction is measured by a remote-sensing video-camera system. The comparison with data is presented as mean absolute relative errors of wave height and mean absolute difference of wave direction. Both wave models produced similar results, but neither could accurately describe waves observed in the physical model in inlets and near structures. The mean absolute relative error of wave height prediction from models was between 22 and 40% as compared with the measured data. The mean absolute error of wave direction estimates ranged from 5 to 12 degrees. Overall, wave direction estimates from GHOST in inlets and near structures compared slightly better with measurements.

DOI: 10.1061/(ASCE)0733-950X(2005)131:4(149)

CE Database subject headings: Inlets; waterways; Wave spectra; Hydrologic models; Wave action; Wave measurement; Fluid-structure interaction.

Introduction

The behavior of waves near an inlet is complicated by their response to shoals, sand bars, breakwaters, and jetties (Penney and Price 1952; Mei 1983; Goda 1985; Massel 1993; Demirebilek and Panchang 1998) and their encounter with tidal currents (Cialone and Kraus, 2001). Waves approaching an inlet generally refract, diffract, and shoal while traveling from deeper water to pass over ebb shoals and propagate through navigation channels and inlets. The change in wave height and direction may significantly affect navigation and sediment movement near coastal inlets. Because measuring waves in coastal inlets is challenging and interferes with navigation, numerical and experimental modeling tools are frequently used in design and maintenance studies. In recent years, spectral wave models that are based on the wave-action conservation equation have become increasingly popular for near-shore wave prediction in coastal inlets. Such numerical models have also been used for wave-structure interaction. A relevant

question is how well this type of model can predict the interaction of nearshore waves with coastal structures.

A number of numerical wave models are available for wave estimates in inlet applications. Models like CGWAVE, GHOST, HISWA, MIKE, STWAVE, and SWAN generally perform similarly and surprisingly well for wave height but perform differently for wave direction (Panchang et al. 1999). Since navigation and sediment transport in the vicinity of inlets are sensitive to wave direction, the predictive capability of numerical wave models must consider the reliability of the estimated wave height and direction. Among the models mentioned, GHOST and STWAVE are the two most similar steady-state spectral wave models for predicting nearshore waves. Both are half-plane models that propagate waves only from the seaward boundary toward the shoreline. Because of their computational efficiency and robustness, both models have been used in a number of coastal engineering studies (<http://www.tecnocan.com> and <http://chl.erdc.usace.army.mil>). These two models were selected and treated as "black boxes" in the present evaluation to determine their suitability for estimating waves in coastal inlets. One of the goals of this evaluation is to provide the engineering community with an objective assessment of these models with measured data sets collected in four different inlet configurations.

GHOST and STWAVE differ in their representation and transformation of the directional wave spectrum (i.e., the distribution of wave energy density in frequency and direction). STWAVE uses a directional spectrum in its wave transformation calculations (Smith et al. 1999, 2001). GHOST uses a marginal directional spectrum for its wave transformation, which is an integrated directional wave spectrum in the frequency range (Rivero et al. 1997a, b). In addition, the treatments of diffraction, reflection, and wave breaking are different in these models. Various

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Note. Discussion open until December 1, 2005. Separate discussions must be submitted for individual papers. To extend the closing date by one month, a written request must be filed with the ASCE Managing Editor. The manuscript for this paper was submitted for review and possible publication on March 16, 2004; approved on December 15, 2004. This paper is part of the *Journal of Waterway, Port, Coastal, and Ocean Engineering*, Vol. 131, No. 4, July 1, 2005. ©ASCE, ISSN 0733-950X/2005/4-149-161/\$25.00.

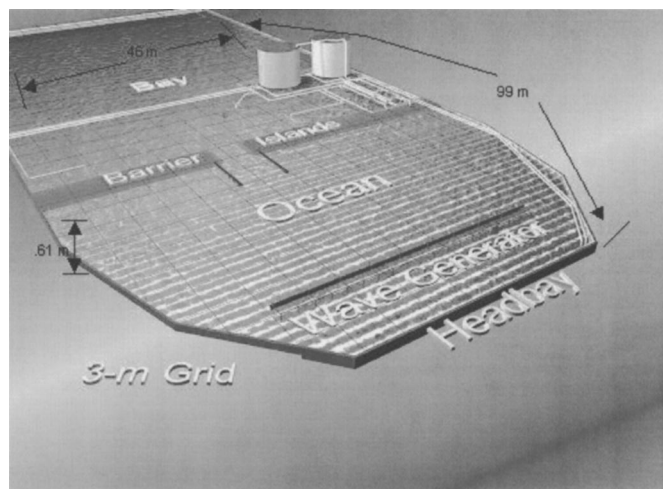


Fig. 1. Idealized inlet model research facility (data from Seabergh et al. 2002).

methods for computing wave reflection and diffraction in the spectral wave models are discussed in a paper by Holthuijsen et al. (2004).

Both models were developed mainly for nearshore wave transformation and are not intended for predicting waves near structures. However, in numerous nearshore applications, coastal structures are commonly present and would affect the wave transformation nearby. It is well known that wave diffraction at the structure is beyond the capability of the wave action conservation equation. This limitation has been addressed by introducing ad hoc modifications of wave-structure interaction in some spectral wave models. These approximate treatments to account for wave diffraction in GHOST have been studied for simple structures on a sloping beach and flat bottom (Carci et al. 2002; Rivero et al. 1997a, b). Similar studies have been conducted for STWAVE (Resio 1993; Smith et al. 1999, 2001). Results of these studies are acceptable for simple structures. However, no comprehensive evaluation of these models has been conducted for complex structures at inlets.

In this study, we have used a new set of wave data collected around an ideal tidal inlet in the laboratory (Seabergh et al. 2002) as a benchmark to examine the overall skills of GHOST and STWAVE models for wave estimation in inlets. This extensive data set helped us reveal shortcomings of the two models with respect to wave structure and wave-current interactions, and helped us identify aspects of the numerical models for further improvement. The laboratory experiments were designed to obtain high-quality wave data in areas where wave-structure interaction is present on a sloping bottom near the jetties and breakwaters and in the bay behind an inlet. Fig. 1 shows the inlet model research facility. Wave-height data were collected by linear arrays of capacitance wave gauges (Fig. 2). The directional spectrum was measured by a unique video-camera system designed to detect the intensity of light reflection on the surface waves (Curtis et al. 2001, 2002). The video-camera system was capable of collecting spatial directional data synoptically over a large area. Similar video techniques have been developed during the past decade and applied in the coastal studies to measure variations in hydrodynamic processes (Holman et al. 1993; Holland et al. 1997; Aarninkhof and Holman 1999). In the present study, the video-camera system was calibrated by using the in situ gauging system for wave height and acoustic-Doppler velocimeters (ADV) for

wave direction. Details of the video-camera system and data analyses are available in the physical model study report by Seabergh et al. (2002).

Numerical Wave Models

STWAVE is a steady-state directional spectral wave model that implements a forward-marching explicit finite-difference method on a square-cell grid to solve the wave action conservation equation (Resio 1981, 1987, 1988, 1993). The model is a half-plane model so that waves can propagate only from the seaward boundary toward the shoreline. Both nonlinear wave-wave interactions and depth/steepness-limited breaking are parameterized as the primary source/sink terms. A prespecified spectrum is required as the incident wave condition at the offshore boundary. The model can accept a homogeneous wind input and simulate wave-current interaction. Wave diffraction is approximately treated and represented in the model by smoothing of strong gradients in wave energy that occur in sheltered areas. The smoothing technique is grid-spacing dependent and uses 55% of energy at a center cell and 22.5% from two neighboring cells (Smith et al. 1999, 2001). Wave reflection and bottom friction are neglected in STWAVE.

GHOST, similar to STWAVE, is also a half-plane and steady-state directional wave spectral transformation model (Rivero et al. 1997a, b; Carci et al. 2002). However, GHOST solves the wave action conservation equation with an implicit finite-difference method on a rectilinear grid. It also requires a prespecified wave spectrum as input at the offshore boundary. The model is capable of simulating wave-structure and wave-current interactions. GHOST can compute wave reflection, diffraction, and wave transmission through and over submerged structures. Bottom friction, wind input, and wave-wave interaction are neglected in the model.

GHOST and STWAVE differ in several aspects. As was previously noted, GHOST adopts a marginal directional spectrum for wave transformation calculations, whereas STWAVE uses the full directional spectrum. GHOST represents wave diffraction by implementing a formulation of the Eikonal equation (Rivero et al. 1997a, b), whereas STWAVE uses a spectral smoothing technique (Resio 1993; Smith et al. 2001). Wave breaking in GHOST is based on the method of Battjes and Janssen (1978). STWAVE uses empirical formulas of depth and steepness limitation (Miche 1944) for its wave breaking. GHOST computes forward wave reflection (in the wave propagation direction) from structures, whereas STWAVE neglects wave reflection. Forward wave reflection is treated in GHOST as a percentage increase of the local incident wave energy at cells in front of a structure. Therefore, with these differences between GHOST and STWAVE, we expected models to produce different results, especially in the vicinity of structures.

Table 1 summarizes the main features of GHOST and STWAVE models. Fig. 3 shows a visual summary of GHOST and STWAVE simulations for waves around a shore-normal groin. This simple test demonstrates how differences in Table 1 between the two models would affect nearshore wave-height estimates near a coastal structure. In this example, the wave-wave interaction in STWAVE was turned off, since GHOST did not have the capability to model wave-wave interaction. The incident wave condition at the ocean boundary is a unidirectional JONSWAP spectrum of 1-m significant height (defined as four times the square root of the total wave energy), with an 8-s peak period and a 20° oblique angle to shore normal. It should be noted that even

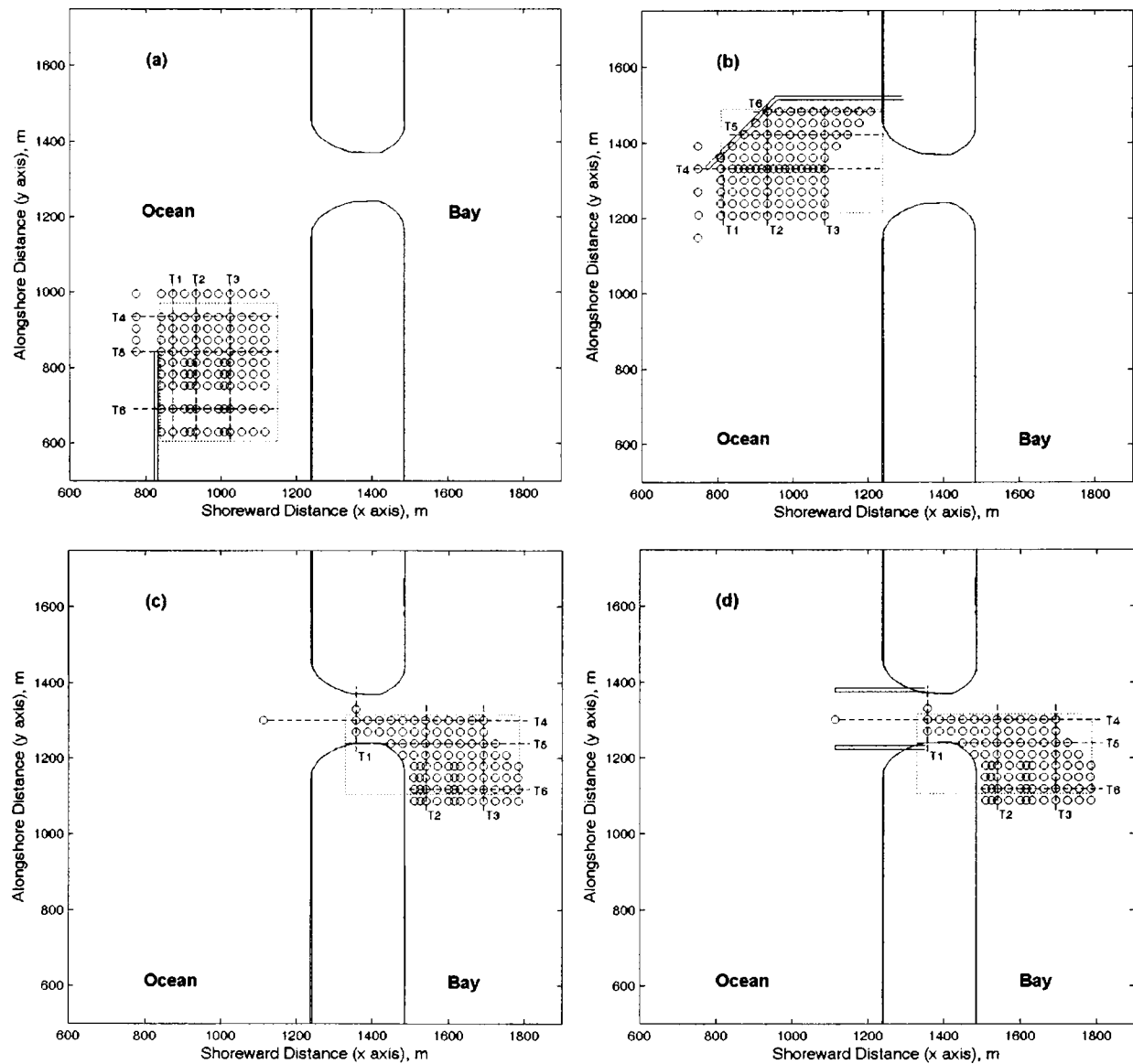


Fig. 2. Location map of wave gauges (circle), rectangular area covered by video-camera system (dotted line), and transect lines (dashed line) for model and measured wave comparisons in the following configurations: (a) S1; (b) S2; (c) S3; and (d) S4

Table 1. Comparison of GHOST and STWAVE Capabilities

Capability	GHOST	STWAVE
Spectrum (half-plane)	Marginal directional	Directional
Refraction/shoaling	Linear	Linear
Diffraction	Equation	Filtering
Wave-current	Nonlinear	Nonlinear
Wave-wave interaction	None	Nonlinear
Wave breaking	Battjes and Janssen (1978)	Depth limitation, Miche (1944)
Reflection	Forward reflection	None
Wave transmission	Linear	None
Wave energy loss by porous bottom	Represented	None
Wave energy loss by bottom friction	None	None
Wave generation	None	Wind input

incident waves are unidirectional and that waves can become multidirectional as they propagate into the shallow water toward the groin as a result of shoaling, refraction, diffraction, wave breaking, and so on. The predictions of GHOST with and without wave reflection are shown in Figs. 3(b and c), respectively. The STWAVE results are shown in Fig. 3(a). A reflection coefficient of 0.3 at the groin (i.e., 30% reflection of wave energy) is used in the results depicted in Fig. 3(c). With wave reflection, GHOST predicts higher wave heights in the upwave side of groin compared with results without wave reflection. STWAVE results show little or no wave diffraction in the lee of the groin and exhibit more wave dissipation on the upwave side of the groin than the GHOST results.

Physical Model

Historically, physical models were used for studying coastal inlets with complex bathymetry and protective structures such as jetties,

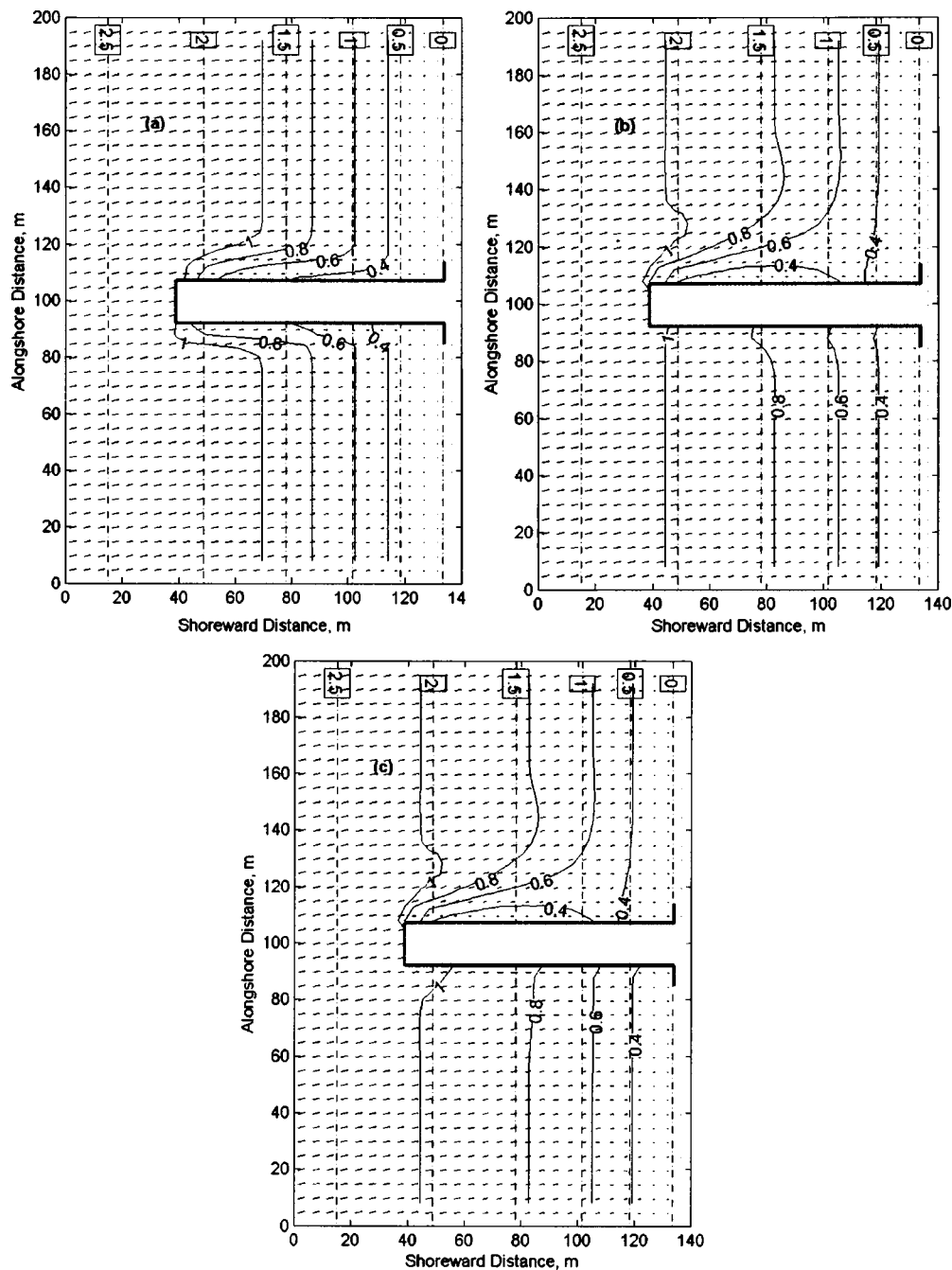


Fig. 3. Model wave results (wave height contours in m, solid line) around an idealized groin (bottom contours in m, dashed line): (a) from STWAVE; (b) from GHOST without reflection; and (c) from GHOST with 30% reflection at the groin

breakwaters, and groins. With advanced techniques in numerical wave models to simulate wind waves, field and laboratory data are now collected primarily to validate such models. Previous laboratory studies of refraction and diffraction were made on a flat bottom, a sloping bottom, and on a flat bottom with a channel incised by two jetties (Harms 1979; Hales 1980; Yu et al. 2000). In these earlier experiments, only wave height was measured. Since waves around coastal structures can turn substantially and change their direction, it was necessary to measure in both wave height and direction in these physical model experiments.

An inlet physical model was designed to measure both wave height and direction (Seabergh 1999; Seabergh et al. 2002) by using a 1:50 undistorted scale model. It was constructed to fit in a

46-m-wide by 99-m-long concrete basin with 0.6-m vertical walls (Fig. 1). This scale replicates a medium-sized Atlantic coast inlet in the United States. The oceanside depth contours were parallel to straight shorelines. The beach slope was specified as an equilibrium profile (Dean 1977), representing approximately a fore-shore slope of 1/15 and an offshore slope of 1/50; and the model extended seaward to the prototype depth of 15 m. The inlet had an average prototype width of 133.4 m. The depth of the inlet throat converged to 7.6 m in prototype at the middle of inlet throat. The bay side floor was flat and had a prototype depth of 6.1 m.

The physical model was capable of generating a steady-state flow to simulate either ebbing or flooding currents by using a

Table 2. Ideal Inlet Experiments (1:50 Scale)

Experiment number	Wave direction (degrees)	Wave period, height, and type	Current on/off
(a) Configuration 1 (offshore breakwater parallel to shore)			
S1X4	20	5.7 s, 3.05 m, irregular wave	Off
S1X5	20	11.3 s, 2.30 m, irregular wave	Off
S1X6	20	5.7 s, 2.30 m, monochromatic wave	Off
(b) Configuration 2 (dogleg jetty at inlet)			
S2X1	0	5.7 s, 3.05 m, irregular wave	Off
S2X2	0	11.3 s, 2.30 m, irregular wave	Off
S2X3	0	5.7 s, 3.05 m, monochromatic wave	Off
S2X4	20	5.7 s, 3.05 m, irregular wave	Off
S2X5	20	11.3 s, 2.30 m, irregular wave	Off
S2X6	20	5.7 s, 2.30 m, monochromatic wave	Off
(c) Configuration 3 (bay measurements, natural inlet)			
S3X1	0	5.7 s, 3.05 m, irregular wave	Off
S3X2	0	11.3 s, 2.30 m, irregular wave	Off
S3X3	0	5.7 s, 2.30 m, monochromatic wave	Off
S3X4	0	5.7 s, 3.05 m, irregular wave	On (flood current)
S3X5	0	11.3 s, 2.30 m, irregular wave	On (flood current)
S3X6	0	5.7 s, 2.30 m, monochromatic wave	On (flood current)
(d) Configuration 4, (bay measurements, dual jetties at inlet)			
S4X1	0	5.7 s, 3.05 m, irregular wave	Off
S4X2	0	11.3 s, 2.30 m, irregular wave	Off
S4X3	0	5.7 s, 2.30 m, monochromatic wave	Off
S4X5	0	11.3 s, 2.30 m, irregular wave	On (flood current)

Note: Maximum flood current is 1.0 m/s at the inlet throat. Various wave/current conditions in the experiment are labeled as X1 to X6.

pipings system. Fig. 1 shows the facility, piping/pump system, and one of the inlet configurations. A flap-hinge 24.4 m-long wave generator was used to produce regular or irregular unidirectional waves. The wave generator was movable so that it could be re-oriented to specify an incident wave direction up to 45° shore-normal. At a Froude model scale of 1:50, prototype waves up to 3 m in height could be generated (Hughes 1993).

The inlet physical model was configured in four idealized “structural” arrangements. In order of presentation, the first is a shore-parallel semiinfinite offshore breakwater (Configuration 1, or S1); the second, a dogleg jetty (Configuration 2, or S2); the third, an inlet through two equal-width barrier islands without a stabilizing structure (Configuration 3, or S3), which is designed to simulate diffraction through a gap; the fourth, a dual-jetty inlet (Configuration 4, or S4) consisting of two equal-length jetties perpendicular to shore. Fig. 2 shows the four configurations in the prototype scale. In Configuration S1, the shore-parallel breakwater is located offshore at the depth contour of 6.8 m in the prototype. The breakwater extends 455 m in the prototype (9.1 m in the physical model) from the sidewall of the physical model. In Configuration S2, the dogleg jetty consists of an inner segment perpendicular to the shore and an outer segment extending seaward at a 45° angle toward the inlet. The prototype lengths of the inner and outer segments are 280 and 265 m, respectively. In S4, two dual jetties, each 170 m long in the prototype, are oriented perpendicular to the straight shorelines of barrier islands.

The experiments for the S1 and S2 configurations were designed for quantifying combined wave diffraction, refraction and shoaling caused by the sloping bottom, jetties, and breakwaters. The experiments for configurations S3 and S4 were designed to

measure wave refraction and diffraction in the bay. Wave conditions used for all configurations consisted of two irregular waves and one regular wave. Two irregular waves represent a short-period wave (mean period of 5.7 s in prototype) and a long-period wave (mean period of 11.3 s). A flood current of the prototype velocity 1 m/s in the entrance channel was created by a piping/pump system in the S3 and S4 experiments to simulate wave-current interaction. Ebbing currents were not tested in these experiments. Table 2 lists the experimental test conditions.

Wave height was measured in situ with 20 wave gauges and remotely with a video-camera system. Wave direction was measured in situ with a pair of ADVs and the video camera system (Curtis et al. 2001, 2002; Seabergh et al. 2002). Wave-height data collected from wave gauges are used in comparing model wave height. Wave direction measured by the ADVs served for checking the quality of the direction data measured by the video camera system. ADV versus video validation statistics for mean wave directions near the spectral peak shows good correlation between the two measurement techniques. Because the premise of the video system depends on the intensity of the reflected light, the measured wave energy density and direction by this system may be less accurate away from the spectral peak. Therefore, in this study, the wave direction at the spectral peak from the video-camera system is used in the comparison with numerical models. The measurement error in the spectral peak vector-mean wave direction is quantified by the standard deviation of the difference (root-mean square, RMS, error) between ADV and video data. This error ranged from 4.6 to 13 degrees for four inlet configurations (Seabergh et al. 2002).

Performance of Numerical Wave Models

In the present study, simulations made with GHOST and STWAVE are based on the same numerical grid. It consisted of 180 (along-shore direction) \times 160 (across-shore direction) square cells. Although the performance of GHOST could be improved by using rectangular cells in areas of high wave gradients (Carci et al. 2002), the STWAVE grid was used in GHOST for a fair comparison of the models. Each cell was 10 \times 10 m in the prototype, and the model grid represented an area of 1.8 \times 1.6 km. The modeling domain extended 940 m to the left of the inlet and 730 m to the right, 790 m offshore to a depth contour of 15 m, and 570 m bayward of the two barrier islands. Inlet width was 130 m (13 cells). A JONSWAP-type unidirectional wave spectrum was input (30 frequency bins and 35 direction bins) at the offshore boundary of both wave models. Standard JONSWAP spectral parameters were used (Goda 1985). The peak enhancement factor (γ) was set to 3.3 and 200 to represent irregular and monochromatic waves, respectively. In the GHOST model, we used a value of 0.1 for the reflection coefficient for jetties and shorelines of the inlet in an attempt to simulate wave reflection observed in the physical model.

Fig. 2 presents the location of wave gauges (circles), the jetties and breakwater structures, the rectangular area (dotted line) covered by the video-camera system, and transects (dashed line) for comparing calculations and measurements. In each modeling configuration, transects T1–T6 were selected for the comparison, with three shore-parallel and three shore-normal transects. For irregular waves, calculated and measured wave heights were the significant height. For monochromatic waves, the mean wave height was used. The modeling condition is denoted by S_nX_m , where $n=1,2,3,4$ (inlet configuration number) and where $m=1,2,3,4,5,6$ (incident wave and current condition number). For example, S1X4 represents the experiment for inlet configuration S1 with incident wave condition X4. Hereafter, wave height refers to significant height for irregular wave input condition and wave direction refers to mean wave direction (calculated as the mean direction from the directional spectrum). For wave-current interaction numerical simulations, the current field was computed from a circulation model ADCIRC (Luettich et al. 1992), which was calibrated with dye tests and which measured ADV currents at the inlet.

Comparisons for Configuration S1

The S1 experiments were designed for diffraction by a detached breakwater and were conducted for three incident wave conditions: an irregular short wave (X4), an irregular long wave (X5), and a monochromatic wave (X6) with 20° oblique incident wave direction to shore-normal for all conditions. Fig. 4 shows the comparison of calculated and measured significant wave heights and mean directions along the six transects (see Fig. 3) for the incident wave condition X4. The RMS error in the measured spectral peak wave direction, quantified by the standard deviation of the difference between ADV and video data, is 4.6° for S1 experiments (Seabergh et al. 2002). GHOST results along T1, T2, and T6 in Fig. 4 are in good agreement with the data, especially in the area behind the breakwater. Otherwise, the performance of the two models is similar. STWAVE predicted wave heights of zero along parts of T1 behind the breakwater. The wave direction associated with these zero wave heights was excluded in the model and data comparison statistics in Tables 3–6. Both models per-

formed poorly for cross-shore transects T3 and T5. Overall, GHOST estimates of wave height and direction are in good agreement with the data for test conditions X4–X6.

Comparisons for Configuration S2

Configuration S2 has a dogleg jetty with the first segment normal to shoreline followed by a second segment aligned approximately 45° from the first segment and pointing toward the inlet. The S2 experiments used six incident wave conditions, X1–X6 (see Table 2). The incident wave direction was shore-normal in X1–X3, and 20° shore-normal in X4–X6. Predicted and measured wave height and direction were compared along six transects (T1–T6), covering the shallow areas shoreward of the jetty (see Fig. 3).

Fig. 5 illustrates the comparison of calculated wave height and wave direction with data along the six transects for the incident wave condition X4. The RMS error in the measured spectral peak wave direction for S2 experiments was 7.0° (Seabergh et al. 2002). Along T1, at the seaward end of the jetty parallel to the shoreline, GHOST and STWAVE show good agreement with wave height and direction data for all incident waves. Both models tend to significantly underpredict wave height along T2–T6, especially in the area behind the jetty where waves are diffracted shoreward by the dogleg jetty. For short-period irregular waves (X1 and X4), STWAVE performed well as compared with data in terms of the predicted wave height. In contrast, for shore-normal long-period irregular waves (X2) and monochromatic waves (X3), GHOST performed well for wave height as compared with the data. For wave direction, GHOST generally performed well for oblique incident waves but was less accurate for shore-normal incident waves.

Comparisons for Configuration S3

Configuration S3 is an ideal natural inlet without any navigation-aiding structures. The physical model experiments consisted of six incident wave and current conditions, all with shore-normal incident wave direction. Test conditions X1–X3 were designed for experiments under the incident waves only in the inlet, whereas conditions X4–X6 included a steady flood flow to study combined wave and current effects. Fig. 6 compares calculated and measured wave height and direction along six transects (T1–T6) located in the bay side of the inlet for test condition X3 (see Fig. 3). The RMS error in the measured spectral peak wave direction for S3 experiments was 13° (Seabergh et al. 2002). Fig. 6 shows the predicted wave height and direction from GHOST; they are in agreement with the data. In this case, STWAVE consistently underpredicted wave heights in the inlet and in the bay. The laboratory experiments indicated a stronger diffraction than model predictions in the bay for all six tested conditions, with or without a flood current.

Comparisons for Configuration S4

The dual-jetty S4 inlet experiments were conducted for four incident wave and current conditions: X1–X3 and X5. The incident wave direction was shore-normal. A sample comparison of the calculated and measured wave height and direction along six transects (T1–T6) located in the bay behind the inlet for X3 is shown in Fig. 7. The RMS error in the measured spectral peak wave direction for S4 experiments was 7.7° (Seabergh et al. 2002). The predicted wave heights from GHOST agreed well with the measurements in Fig. 7, whereas STWAVE underestimated

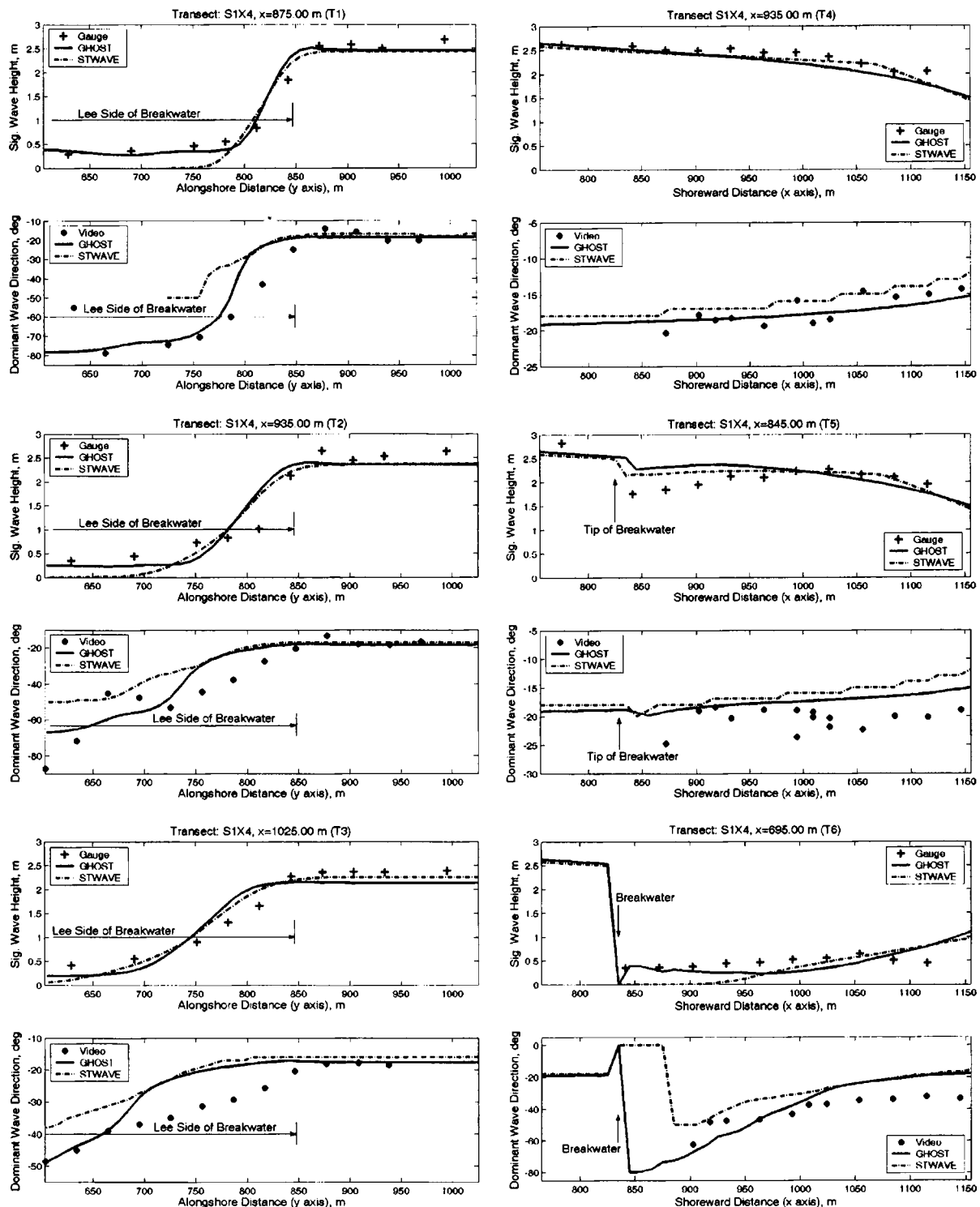


Fig. 4. Model versus measured wave height and direction for S1X4

wave heights along all six transects. GHOST also produced slightly better wave-direction estimates than STWAVE did. For long-period irregular waves (X2), GHOST results for wave heights compared well with the data. However, the model over-predicted wave heights for short-period irregular waves (X1). Both models underestimated wave height for long-period irregular waves with a flood current (X5). STWAVE underestimated wave height along six transects for all four tested incident wave conditions. As compared with the data, both models generally

failed to predict wave direction accurately for Configuration S4. Measurements showed stronger wave diffraction in the bay than wave models did, both with and without a flood current.

Discussion of Results

Two statistical parameters are used in evaluating the overall performance of numerical wave models: (1) the mean of the absolute

Table 3. Statistical Mean Errors of Model Wave Height and Direction for Configuration S1

Experiment number	Mean of absolute relative wave height error (%)	Mean of absolute direction error (degrees)
(a) GHOST		
S1X4	20.2	5.5
S1X5	20.8	5.3
S1X6	24.7	4.6
Average	21.9	5.1
(b) STWAVE		
S1X4	25.4	8.3
S1X5	28.3	6.5
S1X6	35.4	8.3
Average	29.7	7.7

relative error for wave height, defined as the percent change of model and measured wave height (i.e., $100 \text{ percent} \times |\text{predicted} - \text{measured}| / \text{measured}$); and (2) the mean of the absolute difference of model and measured wave direction. These statistical parameters were calculated for each numerical model, and data along transect lines were preselected for each of the four inlet configurations. Since the peak period did not change in either the physical or numerical models, the wave period was not considered in our comparison of model results and data.

Tables 3–6 present statistical comparisons for configurations S1–S4, respectively. The statistics shown are averaged values of all alongshore and cross-shore transects for each experimental condition for a given inlet configuration. The averaging was performed for both wave height and direction. Because STWAVE predicted zero wave height behind the breakwater in Configuration S1, the wave direction associated with the zero height was excluded from the calculated statistics.

Since the performance of the models varied with the incident wave conditions, describing the model performance specifically

Table 4. Statistical Mean Errors of Model Wave Height and Direction for Configuration S2

Experiment number	Mean of absolute relative wave height error (%)	Mean of absolute direction error (degrees)
(a) GHOST		
S2X1	40.8	12.5
S2X2	30.3	8.8
S2X3	33.8	11.5
S2X4	54.6	12.2
S2X5	37.2	7.8
S2X6	40.7	10.2
Average	39.6	10.5
(b) STWAVE		
S2X1	38.9	11.5
S2X2	40.3	9.2
S2X3	37.7	10.7
S2X4	37.8	15.1
S2X5	40.3	11.7
S2X6	31.3	15.1
Average	33.1	12.2

Table 5. Statistical Mean Errors of Model Wave Height and Direction for Configuration S3

Experiment number	Mean of absolute relative wave height error (%)	Mean of absolute direction error (degrees)
(a) GHOST		
S3X1	15.9	6.4
S3X2	35.1	6.3
S3X3	28.5	4.6
S3X4	24.8	6.3
S3X5	19.2	9.4
S3X6	30.9	5.5
Average	25.7	6.4
(b) STWAVE		
S3X1	39.6	7.3
S3X2	36.6	7.0
S3X3	39.1	5.2
S3X4	28.2	6.0
S3X5	25.3	8.8
S3X6	51.3	5.1
Average	36.7	6.6

for regular and irregular waves is necessary. For monochromatic waves (X3 and X6), the errors in wave-height statistics ranged from 24.7 to 40.7% for GHOST and from 35.4 to 51.3% for STWAVE in all inlet configurations (S1 to S4). For short-period irregular waves (X1 and X4) in Configurations S1 and S3, the errors in wave-height estimates were 15.9 to 24.8% versus 25.4 to 39.6%, respectively, for GHOST and STWAVE. In these cases, GHOST seemed to perform better. In contrast, STWAVE performed better for short-period irregular waves (X1 and X4) in S2 and S4, since the errors in wave-height estimates were 34.9 to 54.6% versus 23.4 to 38.9%, respectively, for GHOST and STWAVE.

For long-period irregular waves (X2 and X5), the errors in wave-height estimates varied from 19.2 to 44.5% versus 25.3 to 50.1%, respectively, for GHOST and STWAVE for all configurations (S1 to S4). In this case, GHOST performance was slightly better than STWAVE in predicting wave heights for long-period

Table 6. Statistical Mean Errors of Model Wave Height and Direction for Configuration S4

Experiment number	Mean of absolute relative wave height error (%)	Mean of absolute direction error (degrees)
(a) GHOST		
S4X1	34.9	5.0
S4X2	21.8	9.3
S4X3	18.9	5.7
S4X5	44.5	9.2
Average	30.0	7.3
(b) STWAVE		
S4X1	23.4	5.0
S4X2	34.3	10.1
S4X3	46.3	6.4
S1X5	50.1	9.0
Average	38.5	7.6

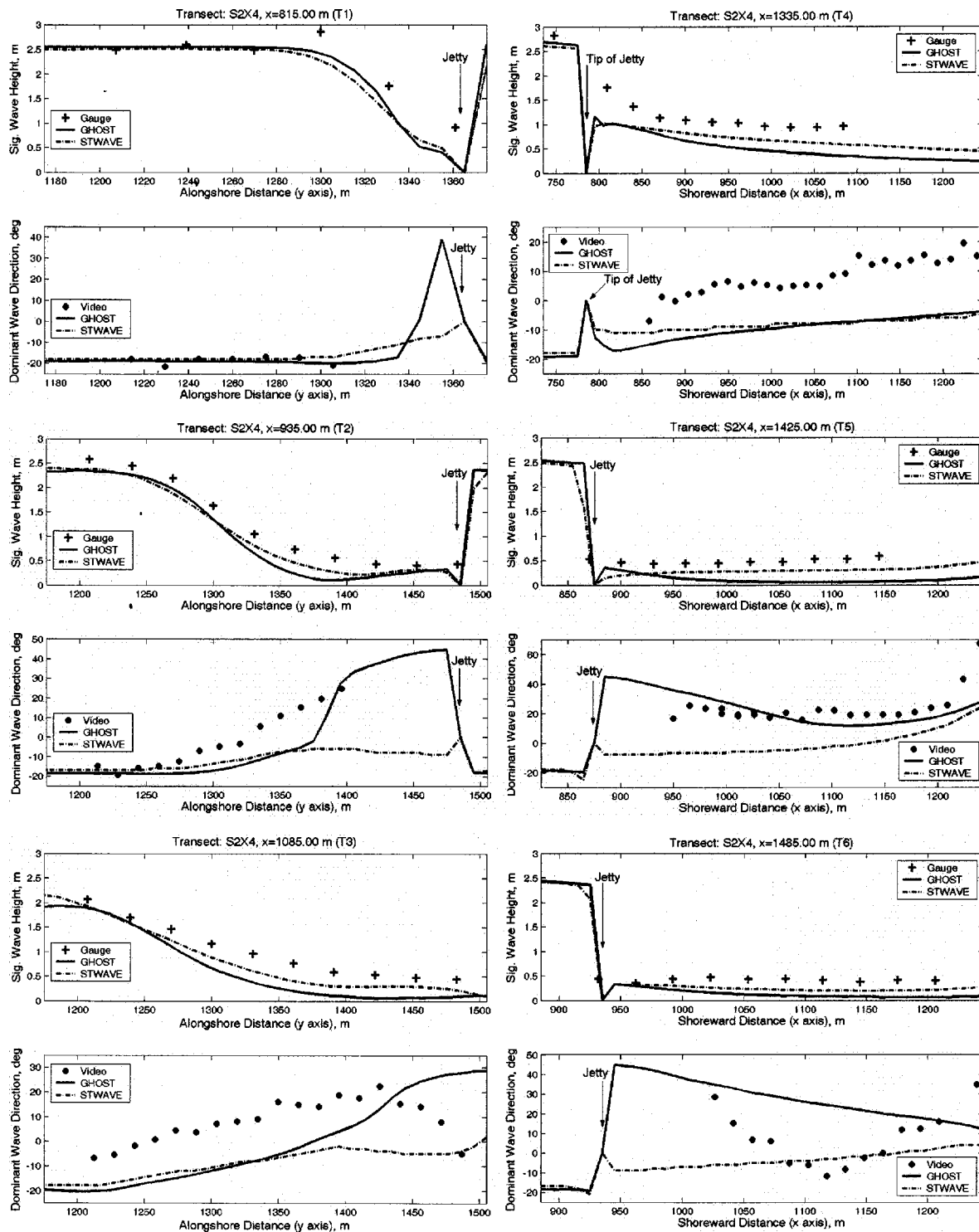


Fig. 5. Model versus measured wave height and direction for S2X4

irregular waves. The overall mean errors in wave height of all test conditions for the four configurations varied from 22 to 40% for GHOST and from 30 to 39% for STWAVE. Therefore, GHOST seems to perform well for wave-height predictions.

The statistics for wave direction (Tables 3–4) indicate that GHOST produced consistently more reliable wave-direction estimates in the lee of jetties and breakwaters (S1 and S2). For Configuration S1, the errors in wave direction varied from 4.6 to 5.5° and from 6.5 to 8.3°, respectively, for GHOST and STWAVE. For S2, the errors were 7.8 to 12.5% for GHOST, and 9.2 to 15.1° for

STWAVE. Both models produced similar wave direction in the inlet channel and in the bay for Configurations S3 and S4. For S3, the errors in wave direction were 4.6 to 9.4° for GHOST versus 5.1 to 8.8° for STWAVE. For S4, the errors were 5.0 to 9.3° for GHOST versus 5.0 to 10.1° for STWAVE. The statistical results of model wave direction for Configurations S1 and S2 are more meaningful, since instrumental error of the video-camera system for these configurations was relatively small (4.6° for S1 and 7° for S2). In contrast, for S3 and S4, the statistics of wave direction are less reliable, since instrumental error (13° for S3 and 7.7° for

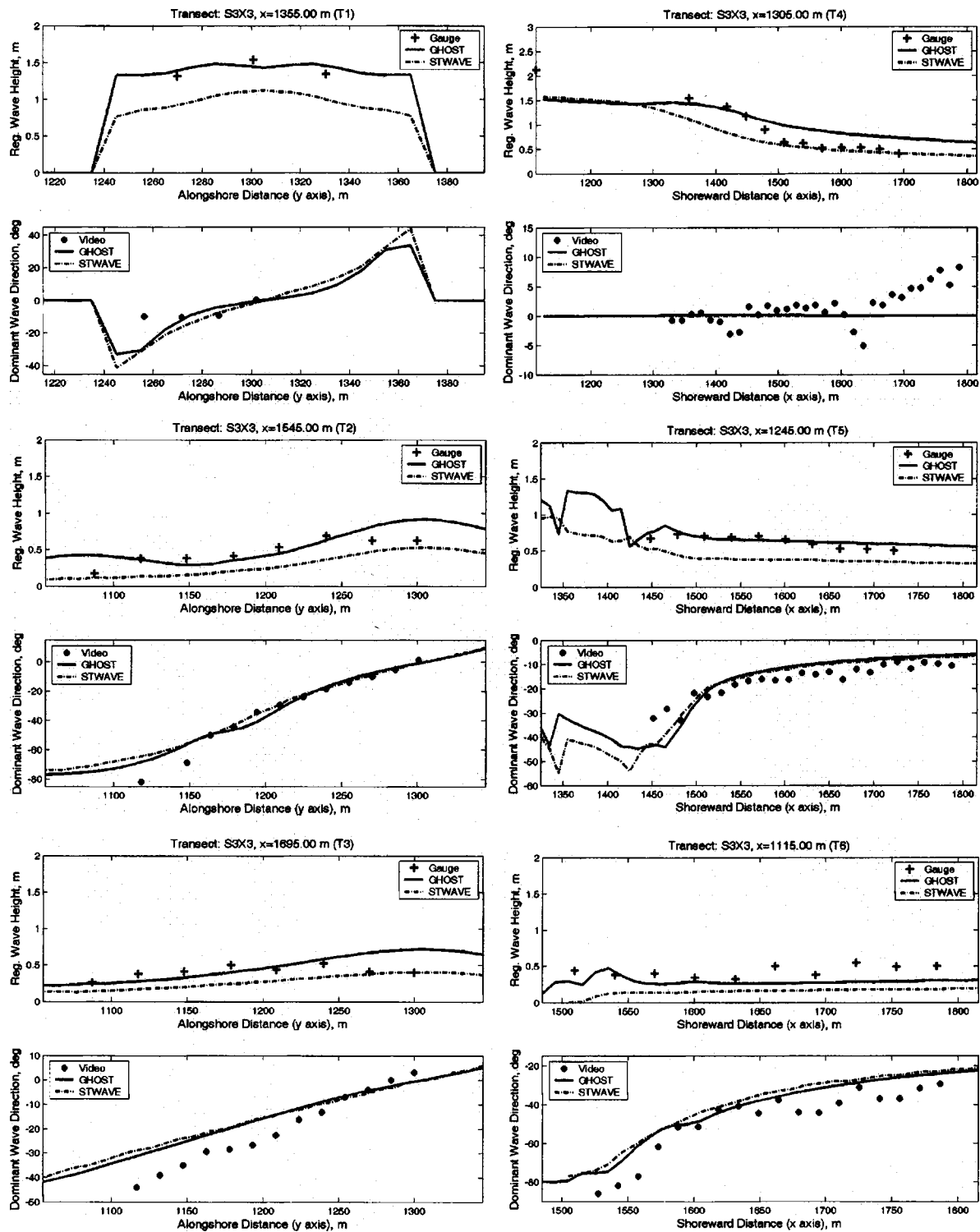


Fig. 6. Model versus measured wave height and direction for S3X3

S4) was equal to or greater than the error in wave-direction estimates.

The difference between model results is mainly attributable to three factors: (1) different representation and transformation of the directional wave spectrum; (2) different treatments of wave diffraction and reflection; and (3) different implementation of wave breaking criteria used in each model. The treatment of wave-current interaction in these models is also a potential cause for differences in model performance. Wave-induced currents (not

simulated in the present study) may contribute to the difference between model results and data. Additional studies are necessary to determine the appropriateness of wave breaking and dissipation implemented in wave models.

The physical model results showed a nonuniform wave-height distribution of waves entering and exiting the inlet that was due to wave shoaling, reflection, refraction, and diffraction. Neither model was capable of capturing these combined wave processes in inlets. Models did not show excessive bending or turning of

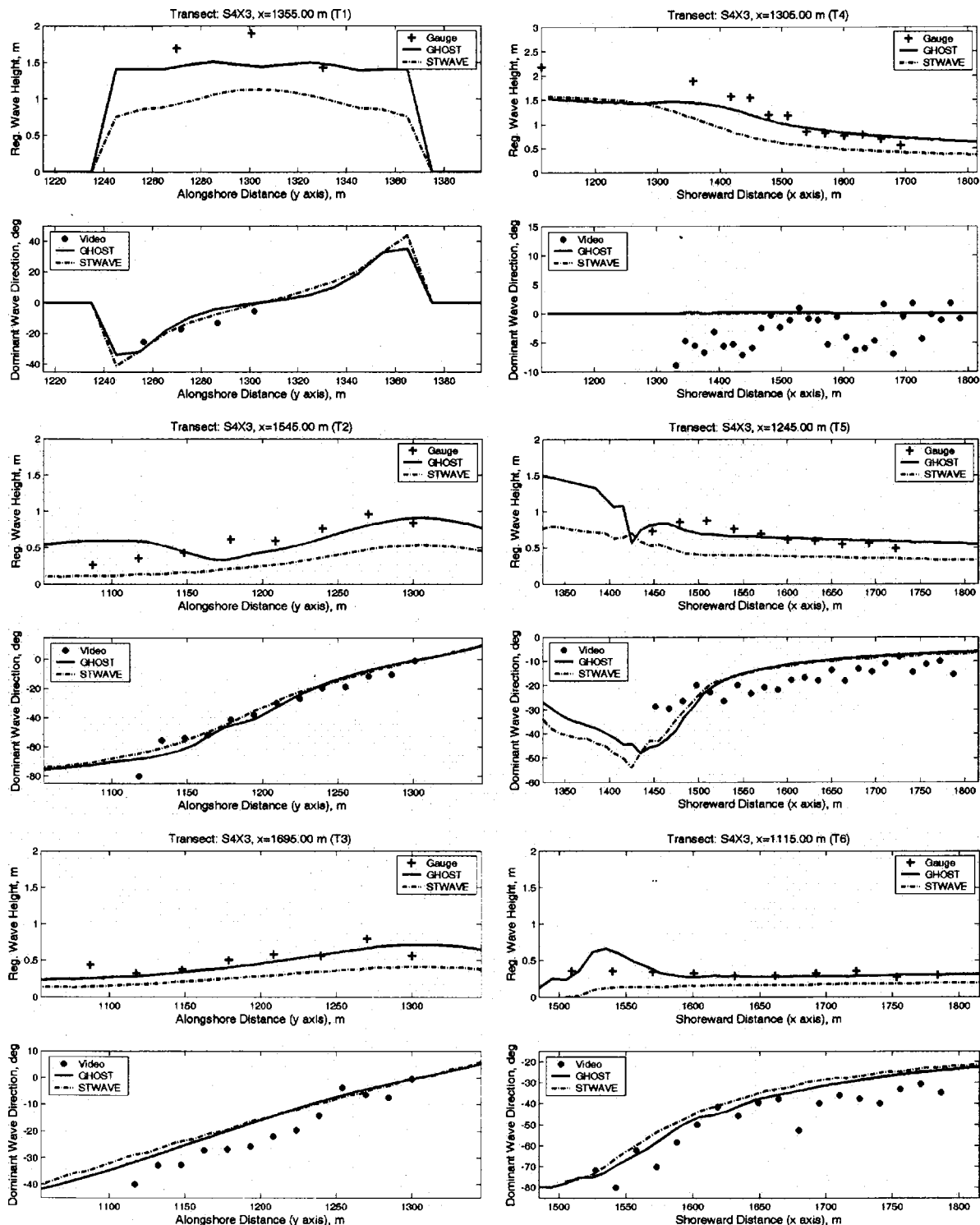


Fig. 7. Model versus measured wave height and direction for S4X3

waves to the extent seen in the physical model tests (Figs. 6 and 7). Waves in the bay dissipated and decayed more rapidly in ST-WAVE simulations, causing the model to consistently underestimate wave heights in the bay. Since GHOST considers wave reflection, it provided slightly better wave-height prediction in the bay.

In the dual-jetty inlet configuration with a flood current (S4X5), both models performed poorly as compared with the data. Models showed excessive dissipation attributable to wave

breaking and wave-current interaction inside the inlet. Models performed similarly for a natural inlet configuration (S3) with a flood current, suggesting that models do not perform well under wave-current interactions in inlets. The model results also showed a significant reduction of wave energy prior to waves entering inlets for both regular and irregular waves (Figs. 6 and 7). This excessive wave-energy reduction can affect model results in the inlet and bay. Such excessive reduction of energy is suspected to be caused by wave-breaking criteria implemented in the models.

Additional studies are necessary to determine the appropriateness of wave-breaking formulas used in models.

Conclusions

The evaluation of the two numerical wave models, GHOST and STWAVE, is presented by comparing their performance with a new and extensive set of laboratory data for four idealized tidal inlet configurations on a sloping beach: a detached breakwater, a dogleg breakwater, a natural inlet (without jetties), and an inlet with dual jetties. We treated the numerical models as “black boxes” in this evaluation, with the objective of determining their predictive skills for estimating waves in coastal inlets. Overall, the performance of the two models was similar, but neither was satisfactory or overwhelmingly better than the other. For the GHOST model, the mean absolute relative error in wave height ranged from 22 to 40% in the four inlet configurations. The error in STWAVE wave-height estimates varied from 30 to 39%. The mean absolute error in wave direction was 5.1 to 10.5° for GHOST and 6.6 to 12.2° for STWAVE. The performance of GHOST showed less error in wave height compared with STWAVE in three of the four inlet configurations (S1, S3, and S4). The mean absolute error in wave-direction by both models was similar in magnitude. Wave-direction estimates by GHOST were generally in better agreement with data.

The emphasis in the present evaluation of GHOST and STWAVE models was focused on the models’ performance around structures (i.e., jetties or breakwaters) at inlets and in the bay. Models were evaluated for four inlet configurations with different incident wave conditions. The errors in wave height and direction from GHOST for monochromatic waves were generally less than those for STWAVE. For irregular waves, the overall performance of GHOST was similar to STWAVE and only slightly better. However, neither model was able to accurately predict waves at inlets in the vicinity of structures and in the bay.

Both models tend to underpredict wave height for all four inlet configurations (Figs. 4–6). Both models were unable to capture the turning of waves in areas where waves interact with currents and structures. The model results showed a noticeable underestimation in the wave height seaward of the inlet. This underestimation may have resulted from neglecting wave scattering and from inadequate treatment of wave-current interaction and wave breaking in inlets. Further investigations of wave breaking and wave-current interaction near inlets are necessary for improving spectral wave models in inlet applications. Since GHOST treats wave diffraction and reflection in a more comprehensive fashion, it is able to provide wave height and direction estimates that are in better agreement with the data. GHOST has additional features for wave transmission over permeable and through porous structures in inlets. STWAVE has additional capability for wind input and wave-wave interaction. These unique features of the models were not investigated in the present study. The comprehensive wave data sets used in this paper are valuable in developing and evaluating of wave models. Work is presently under way to vigorously improve STWAVE capabilities for wave diffraction and reflection.

Acknowledgments

William Seabergh provided the laboratory data and assisted in the model evaluation study. Nicholas Kraus provided encouragement

and helpful comments on this paper. William Seabergh conducted experiments, and provided data and helped in comparison of models with data. This work was performed under the Inlet Modeling System Work Unit of the Coastal Inlets Research Program, U.S. Army Corps of Engineers. Permission to publish this paper was granted by the Chief, USACE.

References

- Aarninkhof, S., and Holman, R. A. (1999). “Monitoring the nearshore with video.” *Backscatter*, 5(11), 8–11.
- Battjes, J. A., and Janssen, J. P. F. M. (1978). “Energy loss and set-up due to breaking of random waves.” *Proc., 16th Int. Coastal Engineering Conf.*, Hamburg, Germany, 569–587.
- Carci, E., Rivero, F. J., Burchart, H. Y., and Maciñeira, E. (2002). “The use of numerical modeling in the planning of physical model test in a multidirectional wave basin.” *Proc., 28th Int. Coastal Engineering Conf.*, Cardiff, U.K., 485–494.
- Cialone, M. A., and Kraus, N. C. (2001). “Wave Transformation at Grays Harbor, WA, with strong currents and large tide range.” *Proc., 4th Int. Symp. of Ocean Wave Measurement and Analysis, Waves 2001*, Vol. 1, ASCE, Reston, Va., 794–803.
- Curtis, W. R., Hathaway, K. K., Holland, K. T., and Seabergh, W. C. (2002). “Video-based direction measurements in a scale physical model.” *Technical Note ERDC/CHL CHETN-IV-49*, U.S. Army Engineer Research and Development Center, Vicksburg, Miss.
- Curtis, W. R., Hathaway, K. K., Seabergh, W. C., and Holland, K. T. (2001). “Measurement of physical model wave diffraction patterns using video.” *Proc., 4th Int. Symp. of Ocean Wave Measurement and Analysis, Waves 2001*, Vol. 1, ASCE, Reston, Va., 23–32.
- Dean, R. G. (1977). “Equilibrium beach profiles: U.S. Atlantic and Gulf Coasts.” *Ocean Engineering Technical Rep. No. 12*, Dept. of Civil Engineering and College of Marine Studies, Univ. of Delaware, Newark, Del.
- Demirbilek, Z., and Panchang, V. (1998). “CGWAVE: A coastal surface-water wave model of the mild-slope equation.” *Technical Rep. CHL-98-26*, U.S. Army Engineer Waterways Experiment Station, Vicksburg, Miss.
- Goda, Y. (1985). *Random seas and design of maritime structures*, University of Tokyo Press, Tokyo.
- Hales, L. Z. (1980). “Erosion control of scour during construction.” *Technical Rep. HL-80-3*, U.S. Army Engineer Waterways Experiment Station, Vicksburg, Miss.
- Harms, V. W. (1979). “Diffraction of waves by shore-connected breakwater.” *J. Hydraul. Div., Am. Soc. Civ. Eng.*, 105(12), 1501–1519.
- Holland, K. T., Holman, R. A., Lippman, T. C., Stanley, J., and Plant, N. (1997). “Practical use of video imagery in nearshore oceanographic field studies.” *IEEE J. Ocean. Eng.*, 22(1), 81–92.
- Holman, R. A., Sallenger, A. H., Lippmann, T. C., and Haines, J. W. (1993). “The application of video image processing to the study of nearshore processes.” *Oceanography*, 6(3), 78–85.
- Holthuijsen, L. H., Herman, A., and Booij, N. (2004). “Phase-decoupled refraction-diffraction for spectral wave models.” *J. Coastal Engineering*, 49, 291–305.
- Hughes, S. A. (1993). *Physical models and laboratory techniques in coastal engineering*, World Scientific, River Edge, N.J.
- Luetich, R. A., Westerink, J. J., and Scheffner, N. W. (1992). “ADCIRC: An advanced three-dimensional circulation model for shelves, coasts and estuaries; Report 1: Theory and methodology of ADCIRC-2DDI and ADCIRC-3DL.” *Technical Rep. DRP-92-6*, U.S. Army Engineer Waterways Experiment Station, Vicksburg, Miss.
- Massel, S. R. (1993). “Extended refraction-diffraction equation for surface waves.” *J. Coastal Engineering*, 19, 97–126.
- Mei, C. C. (1983). *The applied dynamics of ocean surface waves*, Wiley, New York.

- Miche, R. (1944). "Undulatory movements of the sea in constant and decreasing depth." *Ann. Ponts Chaussees*, 25–78, 131–164, 270–292, 369–406 (in French).
- Panchang, V., Xu, B., and Demirbilek, Z. (1999). "Chapter 4: Wave prediction models for coastal engineering applications." *Developments in Offshore Engineering*, J. B. Herbich, ed., Gulf Pub., Houston, 163–194.
- Penney, W. G., and Price, A. T. (1952). "The diffraction theory of sea waves by breakwaters, and shelter afforded by breakwaters." *Philos. Trans. R. Soc. London, Ser. A*, 244, 236–253.
- Resio, D. T. (1981). "The estimation of wind-wave generation in a discrete spectral model." *J. Phys. Oceanogr.*, 11, 510–525.
- Resio, D. T. (1987). "Shallow water waves. I: Theory." *J. Waterw., Port, Coastal, Ocean Eng.*, 113, 264–281.
- Resio, D. T. (1988). "Shallow water waves. II. Data Comparisons." *J. Waterw., Port, Coastal, Ocean Eng.*, 114, 50–65.
- Resio, D. T. (1993). "STWAVE: Wave propagation simulation theory, testing, and application." *Tech. Rep.*, Florida Institute of Technology, Melbourne, Fla.
- Rivero, F. J., Arcilla, A. S., and Carci, E. (1997a). "Implementation of diffraction effects in the wave energy conservation equation." *IMA Conf. on Wind-Over-Waves Couplings: Perspectives and Prospects*, April 8–10, 1997, The Univ. of Salford, U.K.
- Rivero, F. J., Arcilla, A. S., and Carci, E. (1997b). "An analysis of diffraction in spectral wave models." *Proc., 3rd Int. Symp. of Ocean Wave Measurement and Analysis, Waves 97*, Vol. 1, ASCE, Reston, Va., 431–445.
- Seabergh, W. C. (1999). "Physical model for coastal inlet entrance studies." *Coastal Engineering Technical Note CETN-IV-19*, U.S. Army Engineer Research and Development Center, Vicksburg, Miss.
- Seabergh, W. C., Curtis, W. R., Thomas, L. J., and Hathaway, K. K. (2002). "Physical model study of wave diffraction-refraction at an idealized inlet." *Coastal Inlet Research Program, Technical Rep. ERDC/CHL-TR-02-27*, U.S. Army Engineer Research and Development Center, Vicksburg, Miss.
- Smith, J. M., Resio, D. T., and Zundel, A. K. (1999). "STWAVE: Steady-state spectral wave model." *Instruction Rep. CHL-99-1*, U.S. Army Engineer Waterways Experiment Station, Vicksburg, Miss.
- Smith, J. M., Sherlock, A. R., and Resio, D. T. (2001). "STWAVE: Steady-state spectral wave model user's manual for STWAVE." *Version 3.0. Special Rep. ERDC/CHL SR-01-1*, U.S. Army Engineer Research and Development Center, Vicksburg, Miss.
- Yu, Y.-X., Liu, S.-X., Li, Y. S., and Wai, O. W. H. (2000). "Refraction and diffraction of random waves through breakwater." *Ocean Eng.*, 27, 489–509.